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JOB TITLE Example 3 - 157' Building, flat terrain

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CS09 Ver 10.01.10

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STRUCTURAL CALCULATIONS

FOR

Example 3 - 157' Building, flat terrain

Guide to Wind Load Procedures of ASCE 7-02

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Code Search

Code: ASCE 7 - 02

Occupancy:

Occupancy Group = B Business

Occupancy Category & Importance Factors:

Occupancy Category = II
 Wind factor = 1.00
 Snow factor = 1.00
 Seismic factor = 1.00

Type of Construction:

Fire Rating:
 Roof = 0.0 hr
 Floor = 0.0 hr

Building Geometry:

Roof angle (θ) 0.00 / 12 0.0 deg
 Building length (L) 200.0 ft
 Least width (B) 100.0 ft
 Mean Roof Ht (h) 157.0 ft
 Parapet ht above grd 160.0 ft
 Minimum parapet ht 3.0 ft

Live Loads:

Roof 0 to 200 sf: 20 psf
 200 to 600 sf: 24 - 0.02Area, but not less than 12 psf
 over 600 sf: 12 psf

Floor

Typical Floor 50 psf
 Lobbies & first floor corridors 100 psf
 Corridors above first floor 80 psf
 Mechanical 100 psf
 Stairs & Exitways 100 psf
 Balcony / Deck 50 psf
 Partitions 20 psf

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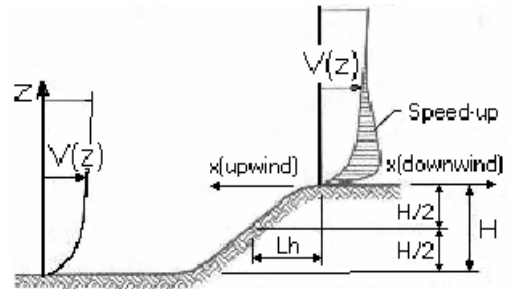
Wind Loads :

Importance Factor 1.00
Basic Wind speed 120 mph
Directionality (Kd) 0.85
Exposure Category B
Enclosure Classif. Partially Enclosed
Internal pressure +/-0.55
Kh case 1 1.124
Kh case 2 1.124
Type of roof Monoslope

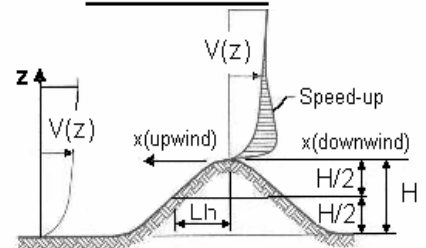
Topographic Factor (Kzt)

Topography Flat
Hill Height (H) 80.0 ft
Half Hill Length (Lh) 100.0 ft
Actual H/Lh = 0.80
Use H/Lh = 0.50
Modified Lh = 160.0 ft
From top of crest: x= 50.0 ft
Bldg up/down wind? downwind

H/Lh= 0.50 K₁ = 0.000
x/Lh = 0.31 K₂ = 0.792
z/Lh = 0.98 K₃ = 1.000
At Mean Roof Ht:
Kzt = (1+K₁K₂K₃)² = 1.00



ESCARPMENT



2D RIDGE or 3D AXISYMMETRICAL HILL

Gust Effect Factor

h = 157.0 ft
B = 100.0 ft
/z (0.6h) = 94.2 ft

Flexible structure if natural frequency < 1 Hz (T > 1 second).
However, if building h/B < 4 then probably rigid structure (rule of thumb).
h/B = 1.57 Therefore, probably rigid structure

G = 0.83 Using rigid structure formula

Rigid Structure
/ε = 0.33
l = 320 ft
z_{min} = 30 ft
c = 0.30
g_Q, g_v = 3.4
L_z = 453.9 ft
Q = 0.83
I_z = 0.25
G = 0.83

Flexible or Dynamically Sensitive Structure
Natural Frequency (n₁) = 0.0 Hz
Damping ratio (β) = 0
/b = 0.45
/α = 0.25
V_z = 102.9
N₁ = 0.00
R_n = 0.000
R_h = 28.282 η = 0.000 h = 157.0 ft
R_B = 28.282 η = 0.000
R_L = 28.282 η = 0.000
g_R = 0.000
R = 0.000
G = 0.000

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Enclosure Classification

Test for Enclosed Building: A building that does not qualify as open or partially enclosed.

Test for Open Building: All walls are at least 80% open.
 $A_o \geq 0.8A_g$

Test for Partially Enclosed Building:

Input		Test	
Ao	0.0 sf	$A_o \geq 1.1A_{oi}$	YES
Ag	0.0 sf	$A_o > 4' / 0.01A_g$	NO
Aoi	0.0 sf	$A_{oi} / A_{gi} \leq 0.20$	NO
Agi	0.0 sf		

Building is NOT Partially Enclosed.

Conditions to qualify as Partially Enclosed Building. Must satisfy all of the following:

- Ao >= 1.1Aoi
- Ao > smaller of 4' or 0.01 Ag
- Aoi / Agi <= 0.20

Where:

- Ao = the total area of openings in a wall that receives positive external pressure.
- Ag = the gross area of that wall in which Ao is identified.
- Aoi = the sum of the areas of openings in the building envelope (walls and roof) not including Ao.
- Agi = the sum of the gross surface areas of the building envelope (walls and roof) not including Ag.

Reduction Factor for large volume partially enclosed buildings (Ri) :

If the partially enclosed building contains a single room that is unpartitioned , the internal pressure coefficient may be multiplied by the reduction factor Ri.

Total area of all wall & roof openings (Aog): 0 sf
 Unpartitioned internal volume (Vi) : 0 cf
 $R_i = 1.00$

Altitude adjustment to constant 0.00256 :

Altitude = 0 feet Average Air Density = 0.0765 lbm/ft³
 Constant = 0.00256

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Wind Loads - MWFRS all h (Enclosed/partially enclosed only)

Kh (case 2) = 1.12	h = 157.0 ft	GCpi = +/-0.55
Base pressure (qh) = 35.2 psf	ridge ht = 157.0 ft	G = 0.83
Roof Angle = 0.0 deg	L = 200.0 ft	z for qi : 157.0 ft use 90.00
Roof tributary area - (h/2)*L: 15700 sf	B = 100.0 ft	qi = 30.0 psf for positive internal pressures
(h/2)*B: 7850 sf		

Surface Pressures (psf)	Wind Normal to Ridge (psf)				Wind Parallel to Ridge (psf)				
	B/L = 0.50		h/L = 1.57		L/B = 2.00		h/L = 0.79		
Surface	Cp	qhGCp	w/+qiGCpi	w/-qhGCpi	Dist.*	Cp	qhGCp	w/+qiGCpi	w/-qhGCpi
Windward Wall (WW)	0.80	23.5	see table below			0.80	23.5	see table below	
Leeward Wall (LW)	-0.50	-14.7	-31.2	4.7		-0.30	-8.8	-25.3	10.6
Side Wall (SW)	-0.70	-20.6	-37.1	-1.2		-0.70	-20.6	-37.1	-1.2
Leeward Roof (LR)	**				Included in windward roof				
Windward Roof: 0 to h/2*	-1.04	-30.5	-47.1	-11.2	0 to h/2*	-0.98	-28.8	-45.3	-9.4
> h/2*	-0.70	-20.6	-37.1	-1.2	h/2 to h*	-0.79	-23.1	-39.6	-3.7
					h to 2h*	-0.61	-18.0	-34.6	1.3

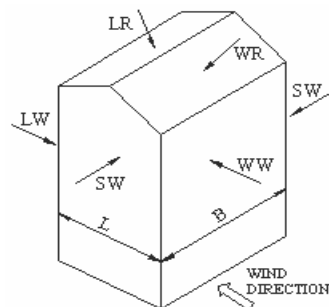
**Roof angle < 10 degrees. Therefore, leeward roof is included in windward roof pressure zones.

*Horizontal distance from windward edge

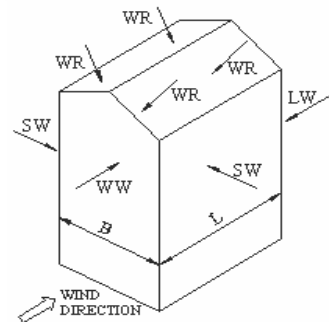
For monoslope roofs, entire roof surface is either windward or leeward surface.

Windward Wall Pressures at "z" (psf)

z	Kz	Kzt	Windward Wall			Combined WW + LW	
			qzGCp	w/+qiGCpi	w/-qhGCpi	Normal to Ridge	Parallel to Ridge
0 to 15'	0.57	1.00	12.0 psf	-4.5 psf	31.4 psf	26.7 psf	20.8 psf
30.0 ft	0.70	1.00	14.6	-1.9	34.0	29.3	23.4
50.0 ft	0.81	1.00	16.9	0.4	36.3	31.6	25.7
80.0 ft	0.93	1.00	19.4	2.8	38.7	34.1	28.2
120.0 ft	1.04	1.00	21.8	5.2	41.1	36.4	30.6
h= 157.0 ft	1.12	1.00	23.5	7.0	42.9	38.2	32.3



WIND NORMAL TO RIDGE



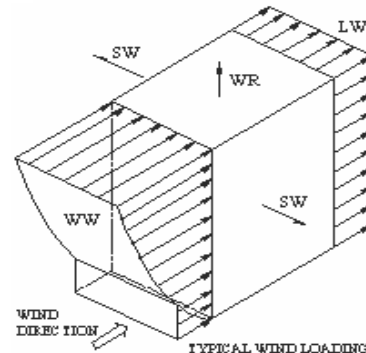
WIND PARALLEL TO RIDGE

NOTE:
See figure 6-9 of ASCE7 for the application of full and partial loading of the above wind pressures. There are 4 different loading cases.

Parapet

z	Kz	Kzt	qp (psf)
160.0 ft	1.13	1.00	35.4

Windward parapet: 63.7 psf (GCpn = +1.8)
Leeward parapet: -39.0 psf (GCpn = -1.1)



TYPICAL WIND LOADING

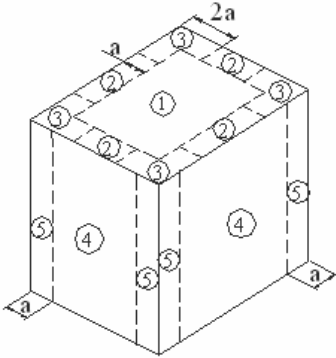
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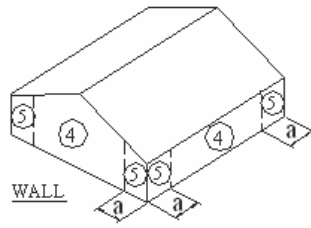
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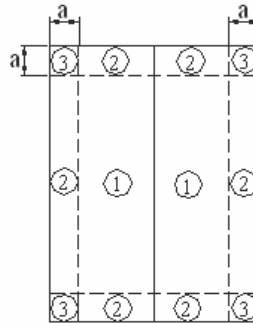
Location of Wind Pressure Zones



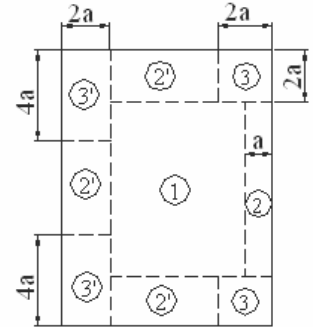
Roofs w/ $\theta \leq 10^\circ$
 and all walls
 $h > 60'$



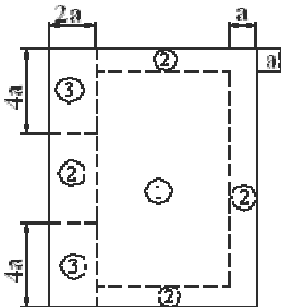
Walls $h \leq 60'$
 & alt design $h < 90'$



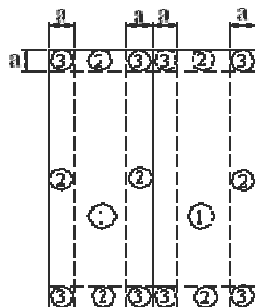
Gable, Sawtooth and
 Multispan Gable $\theta \leq 7$ degrees &
 Monoslope ≤ 3 degrees
 $h \leq 60'$ & alt design $h < 90'$



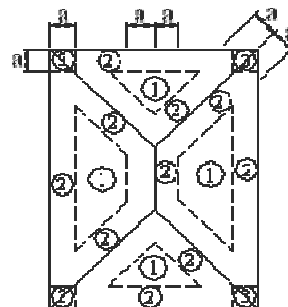
Monoslope roofs
 $3^\circ < \theta \leq 10^\circ$
 $h \leq 60'$ & alt design $h < 90'$



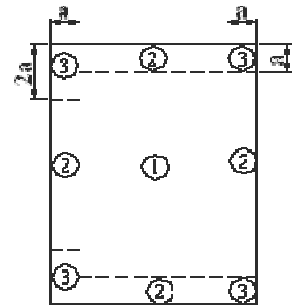
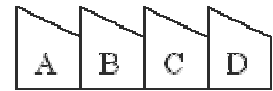
Monoslope roofs
 $10^\circ < \theta \leq 30^\circ$
 $h \leq 60'$ & alt design $h < 90'$



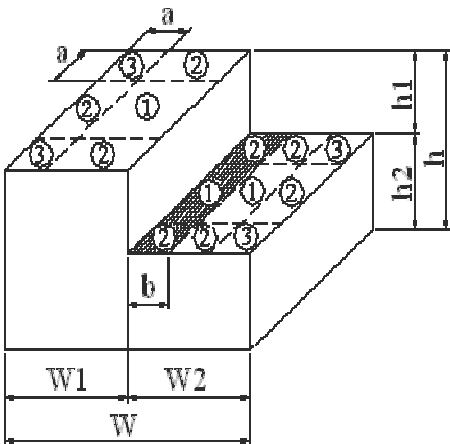
Multispan Gable &
 Gable $7^\circ < \theta \leq 45^\circ$



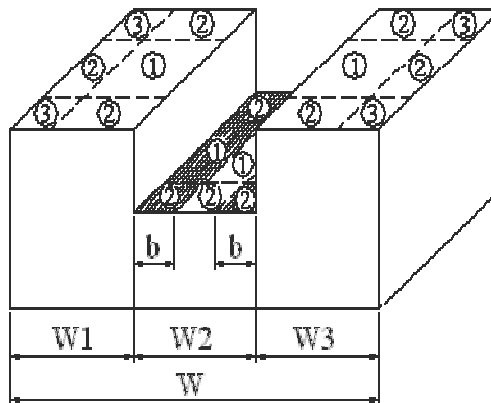
Hip $7^\circ < \theta \leq 27^\circ$



Sawtooth $10^\circ < \theta \leq 45^\circ$
 $h \leq 60'$ & alt design $h < 90'$



Stepped roofs $\theta \leq 3^\circ$
 $h \leq 60'$ & alt design $h < 90'$



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Roof Design Loads

Items	Description	Multiple	psf (max)	psf (min)
Roofing	3 ply felt & gravel		5.5	5.0
Decking	Metal Roof deck, 1.5, 22 ga.		1.7	1.2
Framing	Steel roof joists & girders		3.0	2.0
Insulation	Rigid insulation, per 1"	x 2.0	3.0	1.5
Ceiling	Suspended acoustical tile		1.8	1.0
Sprinklers	Sprinklers		2.0	0.0
Mech & Elec	Mech. & Elec.		2.0	0.0
			0.0	0.0
	Actual Dead Load		<input type="radio"/> 19.0	<input type="radio"/> 10.7
	Use this DL instead		<input checked="" type="radio"/> 20.0	<input checked="" type="radio"/> 9.0
	Live Load		20.0	0.0
	Snow Load		0.0	0.0
	Wind (zone 2 - 100sf)		10.0	-83.0
<u>ASD Loading</u>	Dead + Live Load		40.0	-
	Dead + 0.75(Wind + Live) Load		42.5	-
	0.6*Dead + Wind Load		-	-77.6
<u>LRFD Loading</u>	1.2D + 1.6 Lr + 0.8W		64.0	-
	1.2D + 1.6W + 0.5Lr		50.0	-
	0.9D + 1.6W		-	-124.7

Roof Live Load Reduction

Roof angle 0.00 / 12 0.0 deg

0 to 200 sf: 20.0 psf
 200 to 600 sf: $24 - 0.02 \text{Area}$, but not less than 12 psf
 over 600 sf: 12.0 psf

	300 sf	18.00 psf
	400 sf	16.00 psf
	500 sf	14.00 psf
User Input:	450 psf	15.00 psf

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Floor Design Loads

Items	Description	Multiple	psf (max)	psf (min)
Flooring	Carpet & pad		1.0	1.0
Topping	Concrete regular per 1"	x 4.3	53.1	51.0
Decking	Metal Floor deck - 2", 20ga		2.0	1.5
Framing	Steel floor bms/joists & girders		8.0	5.0
Topping	Deflection Concrete	x 0.8	9.4	1.5
Ceiling	Suspended acoustical tile		1.8	1.0
Sprinklers	Sprinklers		2.0	0.0
Mech & Elec	Mech. & Elec.		2.0	0.0
Misc.	Misc.		0.5	0.0
			0.0	0.0
	Actual Dead Load		79.8	61.0
	Use this DL instead		100.0	50.0
	Partitions		20.0	0.0
	Live Load		50.0	0.0
	Total Live Load		70.0	0.0
	Total Load		149.8	61.0

FLOOR LIVE LOAD REDUCTION (not including partitions)

NOTE: Not allowed for assembly occupancy or LL>100psf or passenger car garages, except may reduce columns 20% if 2 or more floors & non-assembly

$$L = L_o(0.25 + 15/\sqrt{K_{LL}A_T})$$

Unreduced design live load: $L_o = 50$ psf

Floor member $K_{LL} = 2$
Tributary Area $A_T = 300$ sf
Reduced live load: $L = 43.1$ psf

Columns (2 or more floors) $K_{LL} = 4$
Tributary Area $A_T = 500$ sf
Reduced live load: $L = 29.3$ psf

IBC alternate procedure

Smallest of:
 $R = .08\%(SF - 150)$
 $R = 23.1(1+D/L) = 60.0\%$
 $R = 40\%$ beams; 60% columns

$R = 12.0\%$
Reduced live load: $L = 44.0$ psf

$R = 28.0\%$
Reduced live load: $L = 36.0$ psf

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CODE SUMMARY- continued

Component and cladding wind pressures

Roof	Area	Surface Pressure (psf)		
		10 sf	100 sf	500 sf
Negative Zone 1		-65.8	-55.5	-48.2
Negative Zone 2		-97.5	-83.0	-72.9
Negative Zone 3		-97.5	-83.0	-72.9
Positive Zones 1-3		10.0	10.0	10.0

Parapet	Area	Solid Parapet Pressure (psf)		
		10 sf	100 sf	500 sf
CASE A: Interior zone		113.3	93.4	77.9
Corner zone		113.3	93.4	77.9
CASE B: Interior zone		-63.7	-54.9	-46.0
Corner zone		-95.6	-76.1	-56.7

Wall	Area	Surface Pressure (psf)		
		20 sf	100 sf	500 sf
Negative Zone 4		-48.2	-44.7	-41.2
Negative Zone 5		-79.9	-65.8	-51.8
<u>Positive Zone 4 & 5</u>				
0 to 15'		39.1	35.8	32.5
30 ft		39.1	35.8	32.5
50 ft		42.2	38.4	34.6
80 ft		45.5	41.2	36.8
120 ft		48.7	43.8	38.9
h = 157 ft		51.1	45.8	40.5